Mathematical modelling of mobile bed filtration with contact flocculation — filtration arrangement

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In conventional water treatment plants, the unit operations such as rapid mixing, flocculation, sedimentation, filtration and disinfection are used. All these pretreatment units lead to an increase in operational and capital costs. In some cases depending on the raw water characteristics, few of these unit operations can be eliminated. Direct filtration is one of the recent developments in the filtration processes. This filtration system is being designed with only screening, coagulant addition, rapid-mixing and flocculation prior to filtration. Contact flocculation-filtration is a further modification of direct filtration, with only chemical holding tank in the pretreatment unit. Therefore, large capital and operational cost savings can be achieved by this process (Figure 1) if it is technically feasible for the raw water to be treated. The main drawback of this treatment method is the frequent clogging of the filter bed, since the total removal of the suspended matter occurs within the filter bed. Therefore, the filter bed requires frequent backwashing. This frequent filter bed clogging problem during contact flocculation-filtration has led to the development of mobile bed filters. This type of filter arrangement is designed such that the filter grains at the bottom layer are recycled to the top of the filter bed using an air-lift system. Here the recycled filter grains are washed continuously. This process eliminates the problem of backwashing and functions as a continuous filter. There exists several types of design for mobile bed filtration, but, in the case of contact-flocculation filtration process, the particles will be present in the form of flocs, as the flocculation occurs within the filter bed. Therefore the expression for \( \eta \) has to be modified. Table I illustrates the pattern of change in diameter and number of flocs with time.

<table>
<thead>
<tr>
<th>Time</th>
<th>Floc diameter ( d_f )</th>
<th>No of flocs</th>
</tr>
</thead>
<tbody>
<tr>
<td>0</td>
<td>( d_f(0,0) )</td>
<td>( N'(0,0) )</td>
</tr>
<tr>
<td>( \Delta t )</td>
<td>( d_f(L,0) )</td>
<td>( N(L,0) )</td>
</tr>
<tr>
<td>( \Delta t )</td>
<td>( d_f(L,\Delta t) )</td>
<td>( N(L,\Delta t) )</td>
</tr>
<tr>
<td>( \Delta t )</td>
<td>( d_f(L,2\Delta t) )</td>
<td>( N(L,2\Delta t) )</td>
</tr>
<tr>
<td>( \Delta t )</td>
<td>( d_f(L,3\Delta t) )</td>
<td>( N(L,3\Delta t) )</td>
</tr>
<tr>
<td>( (n-1)\Delta t )</td>
<td>( d_f(L,t) )</td>
<td>( N(L,t) )</td>
</tr>
</tbody>
</table>

At a particular filtration time \( t \) and at a fixed filter depth interval \( \Delta L \) all the flocs have same diameter, ie \( d_f(L,t) = \) constant.

The single collector removal efficiency \( \eta \) for deep bed filtration is defined as:

\[
\eta = \frac{\text{Rate at which particles flow towards the collector}}{\text{Rate at which particles strike and attach the filter grain + Rate at which particles strike and attach a particle collector}}
\]

Where \( N \) is the number of retained particles acting as a collector.

The calculation of floc diameter requires an experimental investigation with laboratory-scale flocculation unit with the same raw water to be treated and it is discussed later.

If one assumes, a size distribution of flocs as given in Table I, then the equation for \( \eta \) will be modified as:
Rearrangement of the above equation yields:

\[
\eta_\lambda(L,t) = \eta_\lambda + \frac{a_\lambda a_F}{a_\lambda} \left[ \frac{n_{\eta}(L,t)}{n_{\lambda}(L,t)} \right] \left( \frac{d_{\eta}(L,t)}{d_{\lambda}(L,t)} \right)^2
\]  

Where \( \eta_\lambda \) are the contact efficiencies of a filter grain and floc (particle) collector,
\( a_\lambda \) and \( a_F \) are the attachment coefficients, between filter grain to floc and floc to floc.
\( d_\eta, d_\lambda \) are the diameters of flocs and filter grain.
\( N(L,t) \) is the number of particle collector at depth interval \( L \) to \( L + \Delta L \) and time \( t \).
\( n_F(L,t) \) is the floc number concentration at depth \( L \) and time \( t \).

In the Equation 3 the first term on the R.H.S. represents the removal efficiency of a single filter grain and the second term represents the increase in the removal brought by the flocs attached to the filter grain.

The change of number of particle collectors with time can be calculated from the following equation.

\[
\frac{d N(L,t)}{dt} = a_\eta a_F V n_F(L,t) \cdot \frac{(1/2)}{L^2} \cdot \frac{n_{\eta}(L,t)}{n_{\lambda}(L,t)} \left( \frac{d_{\eta}(L,t)}{d_{\lambda}(L,t)} \right)^2
\]  

where \( \beta \) is the fraction of retained flocs acting as particle collectors.

The number of particle collectors at a given time can be calculated from the following equation which is a partial differential form of the above equation.

\[
N(L,t) = N(L,t - \Delta t) + (n_F(L,t - \Delta t) - n_F(L,t) \cdot V \cdot \Delta t)
\]  

From the mass balance of suspended particles removal the following equation can be obtained:

\[
\frac{d n_F(L,t)}{dt} + V \cdot \frac{d n_F(L,t)}{dt} + (\frac{1}{L^2}) n_F(L,t) \cdot V \cdot \frac{(1 - \beta)}{L^2} \cdot \frac{n_{\eta}(L,t)}{n_{\lambda}(L,t)} \left( \frac{d_{\eta}(L,t)}{d_{\lambda}(L,t)} \right)^2 = 0
\]  

\( \tau_\eta \) and \( \tau_\eta(L,t) \) at different depths and time can be calculated by solving the Equations 3 through 6. Since there is no analytical solution, in this study, these equations were solved considering \( \tau_\eta \) and \( \tau_\eta(L,t) \) as step function of time as have been treated by O'Melia and Ali.'
Kozeny's equation for the calculation of clean bed headloss development is:

$$h_0 = \frac{k_0}{L} = \frac{k}{k_0} \cdot \frac{V}{\varepsilon} \cdot \frac{(1-\varepsilon)^2}{\varepsilon^3} \cdot \frac{\varepsilon^2}{\varepsilon_f^2}$$  

(8)

As the filtration proceeds, the specific surface ($a_g$) is modified due to the deposition of flocs on the filter grains. The change of specific surface can be calculated in the following manner:

$$a_g = a_g + a_f = \frac{A_c}{C} + \frac{A_f}{F}$$  

(9)

For the size distribution of flocs given in the Table 1, the change in specific surface can be written as follows:

$$\Delta a_g = \Delta \left( \frac{A_c}{C} + \frac{A_f}{F} \right)$$

(10)

Where, $\beta$ is the fraction of the total number of retained flocs, that contribute to the additional surface.

Substituting the above value of $a_g$ in Equation 9, the following equation for headloss can be obtained.

$$h_f(L,t) = \frac{36}{L} \cdot k_0 \cdot \frac{V}{\varepsilon} \cdot \frac{(1-\varepsilon)^2}{\varepsilon^3} \cdot \frac{1}{\varepsilon_f^2} \cdot \left[ \frac{1 + \beta \left( 1/N_c \cdot d_c \right)^2 \cdot N_F(L,t) \cdot \left( d_f(L,t) \right)^2}{1 + \beta \left( 1/N_c \cdot d_c \right)^2 \cdot N_F(L,t) \cdot \left( d_f(L,t) \right)^2} \right]^2$$  

(11)

There exists no general relationship between floc size, velocity gradient and concentration of suspended solids. Therefore in the present study, the semi empirical equations (12) and (13) obtained from Jar Test experiments for the kaolin clay suspension in the presence of Cat Floc-T were used to calculate the ultimate diameter of floc, and floc diameter at different time and depth of the filter.

$$d_f(t) = \left( \frac{1234721.01}{G} \right)^{0.8}$$  

(12)

where, $G$ is in sec.

$$d_u$$ is in $\mu m$

The floc diameter at different depths and time is given as:

$$d_f(t) = 10^6 \left[ \log \left( \frac{d_p}{d_0} \right) + 0.0015 \cdot \frac{1}{G} \cdot \frac{1}{t} \right]$$

(13)
The different steps involved in the process of floc diameter calculations are summarized here.

**Calculation of velocity gradient (G):**
The velocity gradient (G) was calculated from the following relationship:

\[ \dot{e} = \frac{P}{\rho V} \]  

(14)

Where \( P \) is the power dissipated within the filter layer, which can be calculated from the following relationship:

\[ P = [\text{sink}(L + \Delta L, L + \Delta L) - \text{sink}(L, L + \Delta L)] \rho g \]  

(15)

Where \( V \) is the filter bed volume of the layer considered. \( Q \) is the flow rate of the suspension.

**Calculation of ultimate floc diameter:**
The ultimate floc diameter was calculated using the \( G \) values calculated and Equation 12 at different depths and times.

**Calculation of floc diameter:** The floc diameter at different depths and times was then computed using Equation 13. It should be noted that the formulation of Equation 13 was based on Boardway’s approach.

**Mobile bed with contact flocculation-filtration**
The developed model for fixed bed filters with contact-flocculation filtration arrangement was modified to mobile bed filters considering the relative approach velocity of suspension with respect to the sand layer movement (\( V_r \)). The calculation process is briefly schematized in Figure 2. The final equations for single collector removal efficiency and headloss in the mobile bed filter are as presented below:

\[ n_k(L, c) = n_k \cdot \left[ 1 + n_k^2 \cdot n_k^2 \cdot c \right] \]

\[ \frac{\partial n}{\partial t} - \frac{\partial (n \cdot c)}{\partial x} = \frac{(N_{-c})}{L} \frac{c}{c} \left[ \frac{\partial (n \cdot c)}{\partial x} \right] \]

(16)

\[ h_2(L, c) = 36 \cdot \frac{k}{g} \cdot \frac{(N_{-c})}{c} \frac{1 - c^2}{c^2} \]  

(17)

Experimental
A set of experiments were carried out with laboratory-scale fixed bed and mobile bed filter models. The mobile bed filter model used in the study is presented in Figure 3. Sand with sizes ranging from 0.841-1.168mm was selected as the filter.
Mobile bed filtration with contact flocculation-filtration arrangement:
As in the case of fixed bed filtration model, the clean bed removal efficiency for the mobile bed filtration model, \( \alpha_0 \), was calculated from the experimental data. Then C/Co profiles at different recycle rate were simulated. It was noticed that the C/Co profiles could not be simulated with the same \( \alpha_0 \) used for fixed-bed filter experiments (Figure 4). This is the main drawback of the modified model.

Conclusions
The O'Melia-Ali's model can be modified for contact-flocculation filtration by taking into account the flocculation phenomena within the filter bed. This requires the development of empirical equations relating the floc size variation with velocity gradient and flocculation time from laboratory-scale flocculation experiments with the same suspension used for filtration. The value \( \alpha_0 \) appearing in the model was found to decrease with the increase in filtration velocity instead of remaining constant. This is a short-coming of this model. The removal efficiency of mobile bed filter (with contact flocculation-filtration arrangement) could not be successfully simulated for different recycle rates from the modified O'Melia and Ali's model incorporating the sand layer movement using the same value of \( \alpha_0 \) calculated for the fixed bed filter under identical operating conditions. A further modification of the model is therefore essential.

References

Coating material
A new low-bake Duraguard P3353 series of electrostatic polyester powder paints is launched which it is claimed can Ideal for environmental applications, many of which have been innovative.

New brochure
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To compliment the Series 3 Sludgepress range, a pre-thickener is also available. Mounted above the main press unit, the pre-thickener will accept the thin sludges and increase the solids content by gravity drainage, so reducing the hydraulic loading on the main press and enabling it to operate at optimum conditions.

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